

# Geostatistics for modeling of soil spatial variability in Adapazari, Turkey

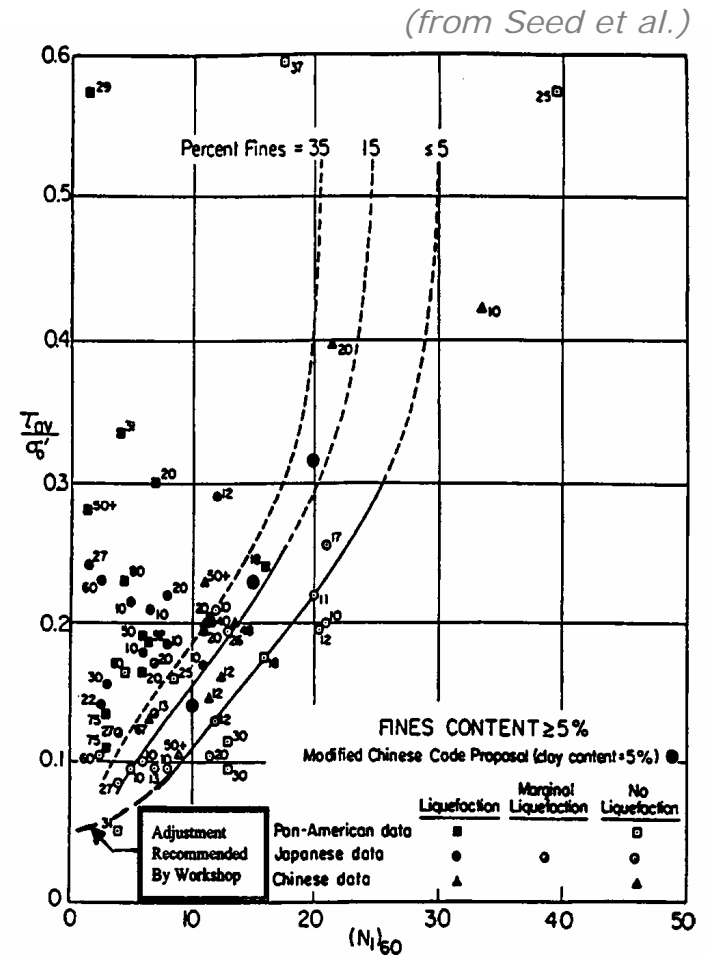
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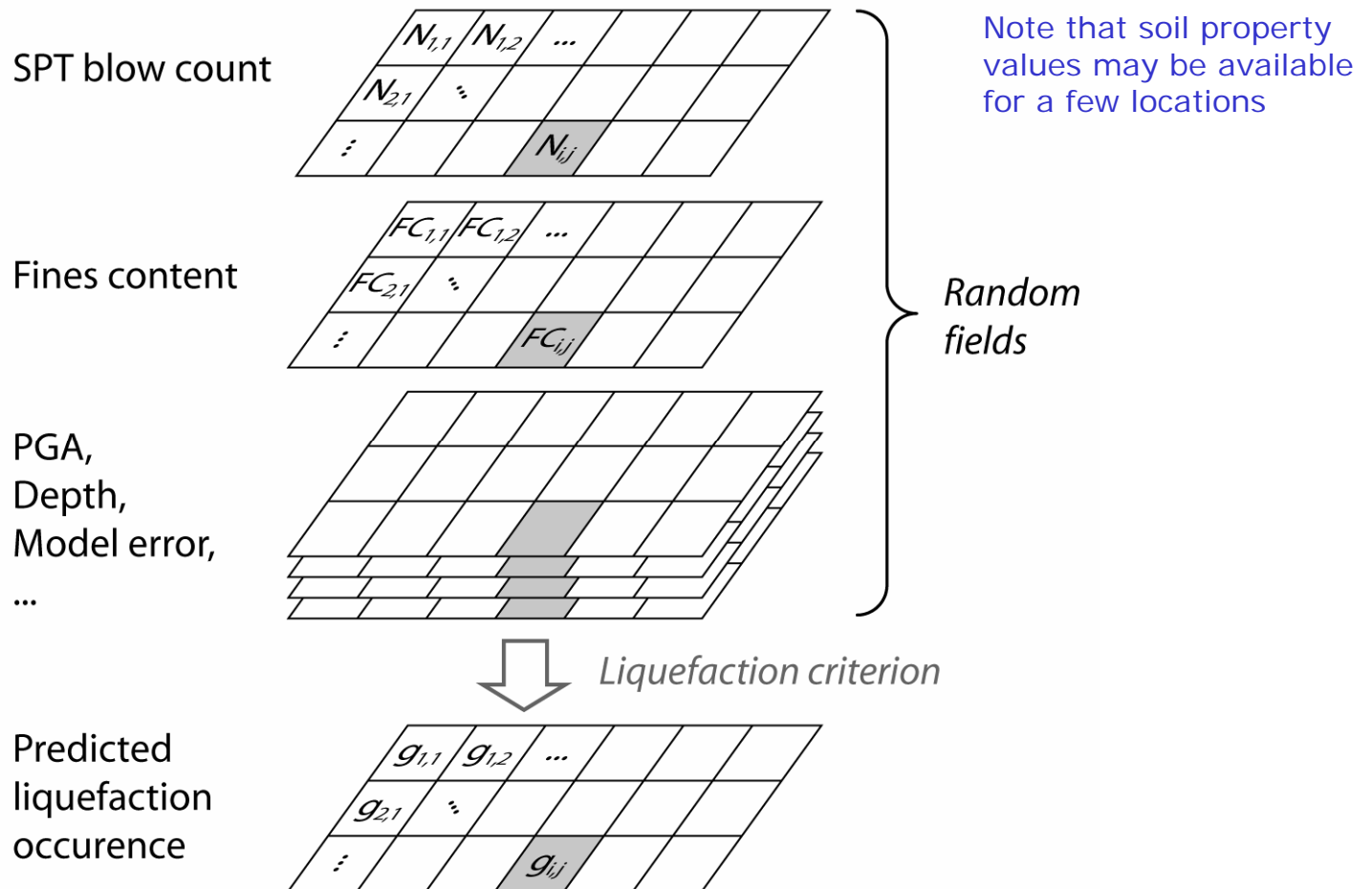
# Practical evaluation of liquefaction occurrence

- Obtained from empirical observations
- Useful for practical evaluations
- Applied only at single locations

An approach is proposed here for incorporating spatial dependence of soil properties to evaluate the potential spatial extent of liquefaction



## Proposed procedure for evaluating liquefaction extent

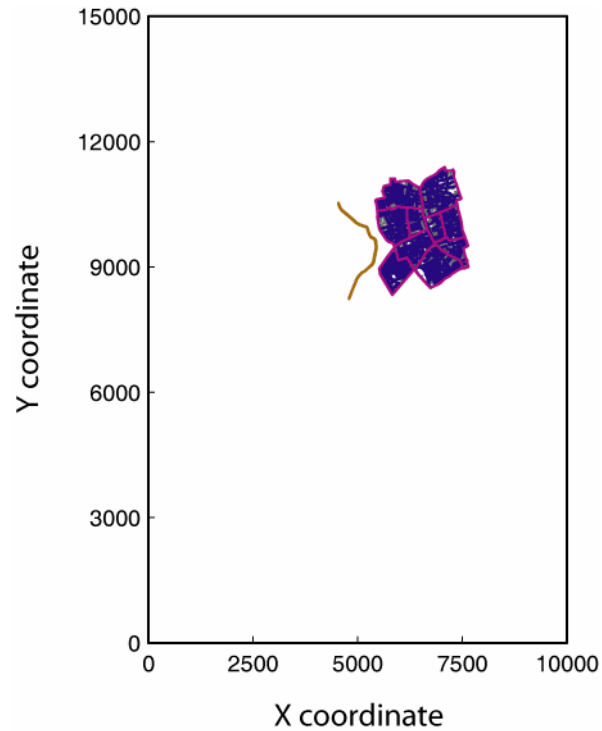


# A demonstration site in Adapazari, Turkey

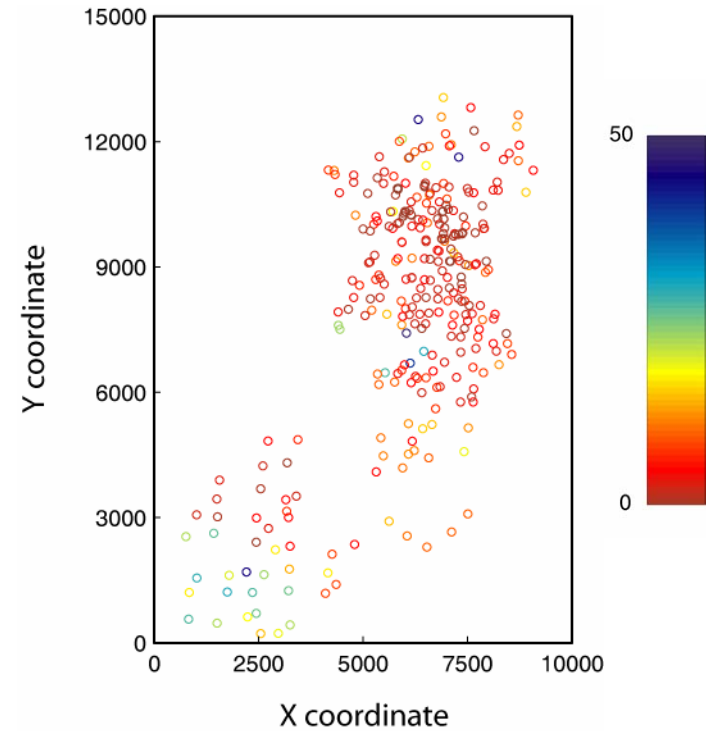


# Sampled $N_{1,60}$ values near the demonstration site in Adapazari, Turkey

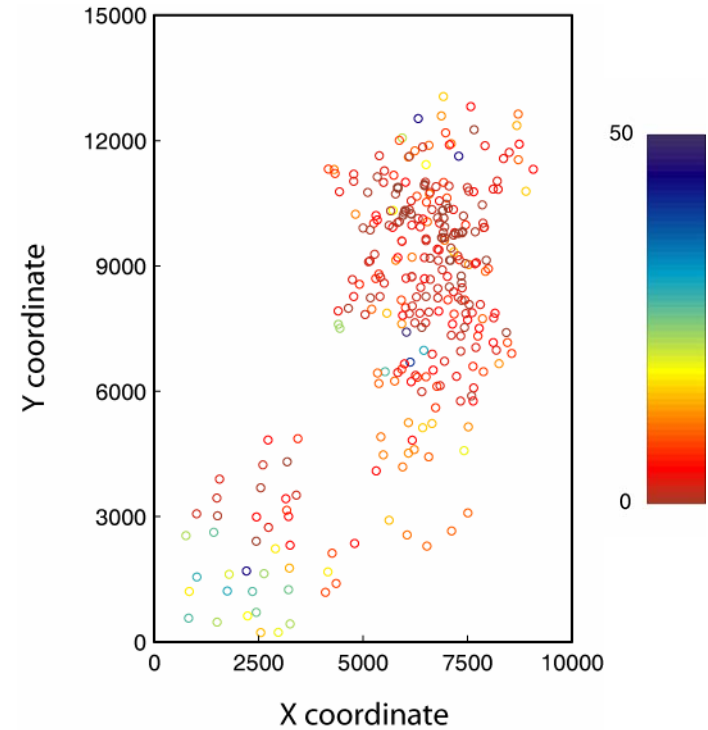
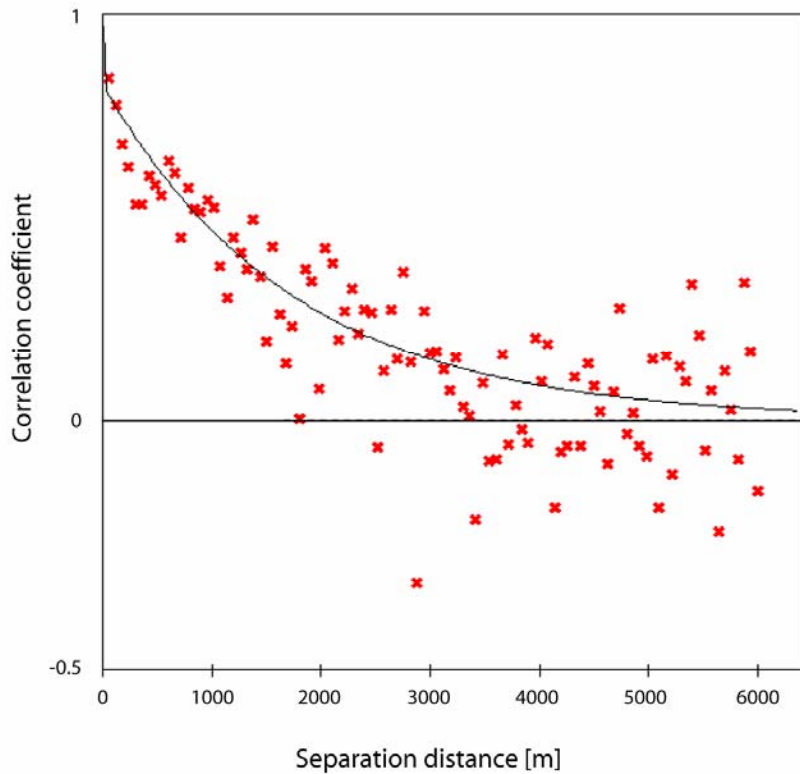
Test site



Sampled soil values



# Sampled $N_{1,60}$ values near the demonstration site in Adapazari, Turkey

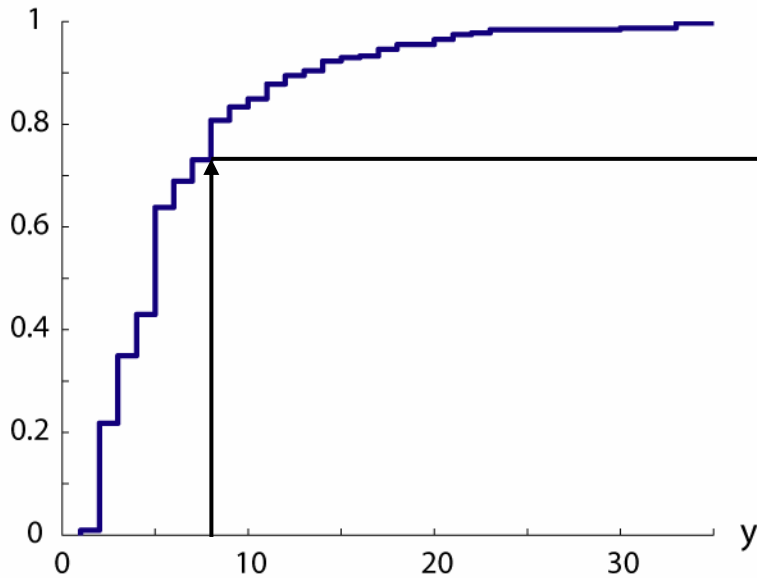


$$\rho(h) = 0.85 \left[ 1 - \exp\left(-h/5000\right) \right]$$

## Simulation of $N_{1,60}$ values, conditional on observations (*Sequential Gaussian Simulation*)

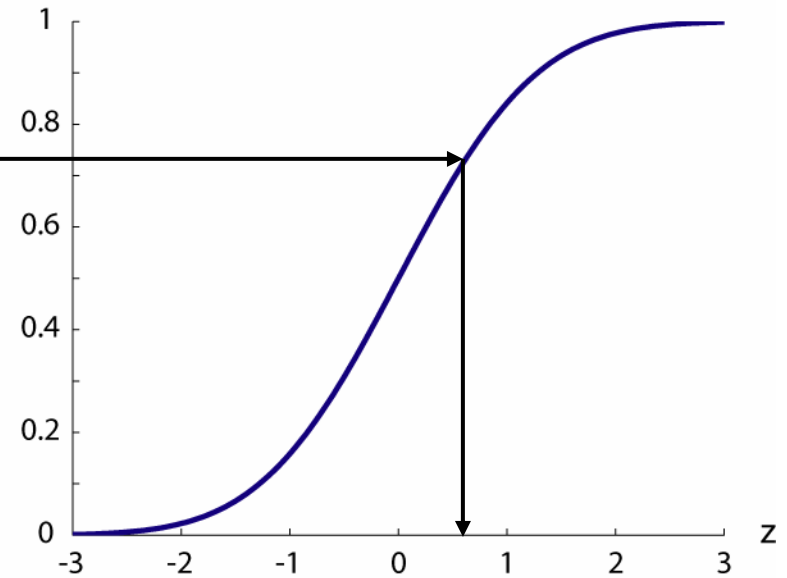
1. Transform data so that it is normally distributed:  $z = \Phi^{-1}(F(y))$

$F(y) = P(Y < y)$



Empirical CDF of  $N_{1,60}$  values

$\Phi(z) = P(Z < z)$



Standard normal CDF

## Simulation of $N_{1,60}$ values, conditional on observations (*Sequential Gaussian Simulation*)

2. Estimate spatial dependence of soil properties

$$\rho(h) = 0.85 \left[ 1 - \exp\left(-\frac{h}{5000}\right) \right]$$



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2. Estimate spatial dependence of soil properties

$$\rho(h) = 0.85 \left[ 1 - \exp\left(-\frac{h}{5000}\right) \right]$$

3. Simulate one additional point ( $Z_1$ ), conditional upon observed values

$$\begin{bmatrix} Z_1 \\ \mathbf{Z}_{orig} \end{bmatrix} \sim N \left( \begin{bmatrix} 0 \\ \mathbf{0} \end{bmatrix}, \begin{bmatrix} 1 & \boldsymbol{\Sigma}_{12} \\ \boldsymbol{\Sigma}_{21} & \boldsymbol{\Sigma}_{22} \end{bmatrix} \right) \quad \longrightarrow \quad (Z_1 | \mathbf{Z}_{orig} = \mathbf{z}) \sim N \left( \boldsymbol{\Sigma}_{12} \cdot \boldsymbol{\Sigma}_{22}^{-1} \cdot \mathbf{z}, 1 - \boldsymbol{\Sigma}_{12} \cdot \boldsymbol{\Sigma}_{22}^{-1} \cdot \boldsymbol{\Sigma}_{21} \right)$$

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4. Repeat step 3 for each location, treating previously simulated points as fixed

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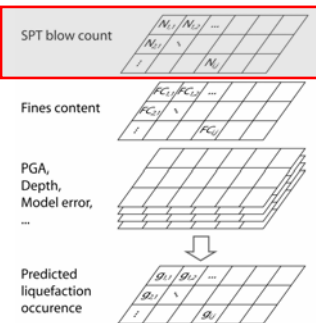
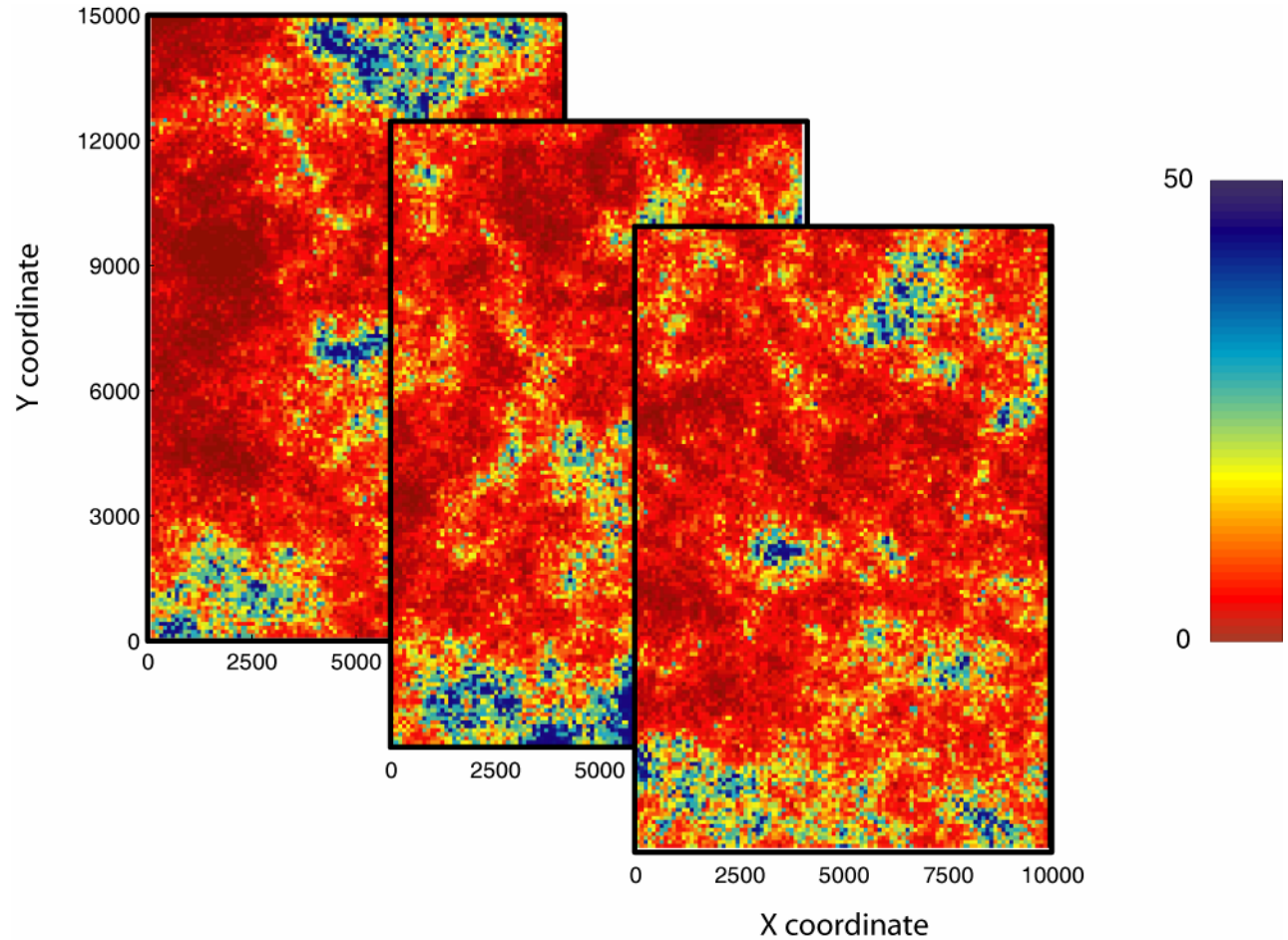
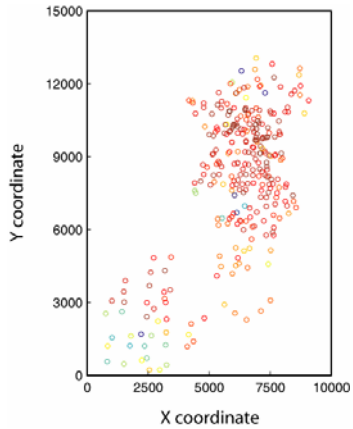
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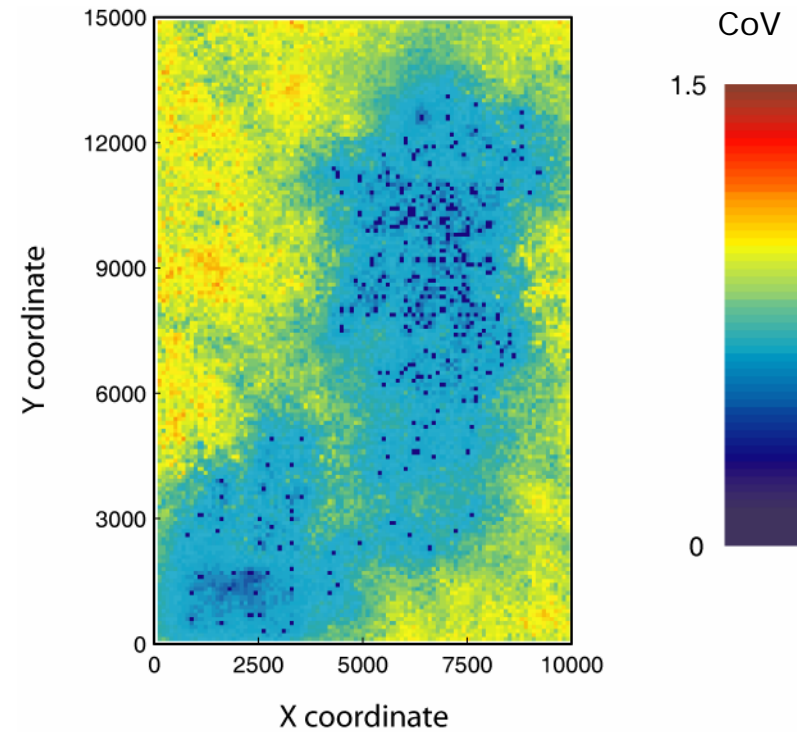
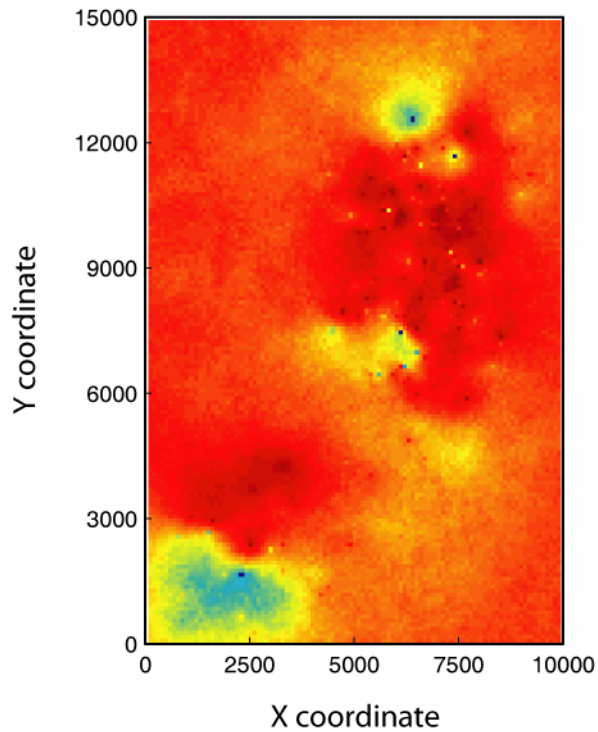
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4. Repeat step 3 for each location, treating previously simulated points as fixed
5. Transform the simulated Gaussian variables back to the original distribution

# Conditional simulations of $N_{1,60}$



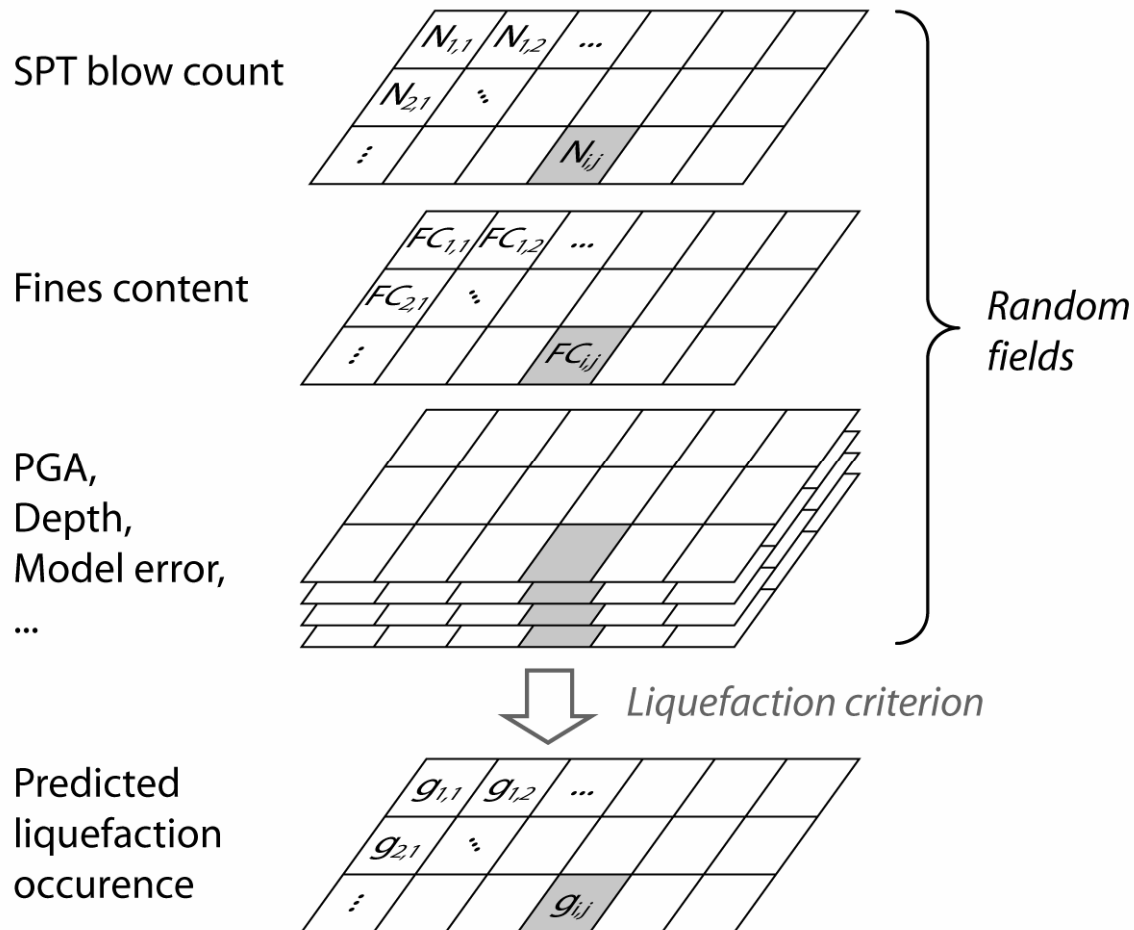
# Mean and coefficient of variation of conditional $N_{1,60}$ simulations



## Comments on Sequential Gaussian Simulation

- Common in petroleum engineering and mining
- Requires jointly Gaussian random variables
  - Partially achieved using the normal score transform
  - This numerical transform is applicable for any probability distribution
- Can also be used for vectors of dependent parameters (e.g., SPT blow count and fines content)
- It is not necessary to condition on all previous points when simulating

# Procedure for evaluating liquefaction extent



## A probabilistic liquefaction criterion (Cetin et al., 2004)

$$g(\mathbf{X}, \mathbf{u}) = N_{1,60} (1 + 0.004 FC) - 13.32 \ln CSR_{eq} - 29.53 \ln M - 3.70 \ln \frac{\sigma'_v}{P_a} + 0.05 FC + 44.97 + \varepsilon_L$$

$$CSR_{eq} = 0.65 \left( \frac{PGA}{g} \right) \left( \frac{\sigma'_v}{\sigma_v} \right) r_d$$

$$r_d = \frac{\left[ 1 + \frac{-23.013 + 2.949 a_{\max} + 0.999 M_w + 0.0525 V_{s,12m}^*}{16.258 + 0.201 e^{0.341(-d + 0.0785 V_{s,12m}^* + 7.586)}} \right]}{\left[ 1 + \frac{-23.013 + 2.949 a_{\max} + 0.999 M_w + 0.0525 V_{s,12m}^*}{16.258 + 0.201 e^{0.341(0.0785 V_{s,12m}^* + 7.586)}} \right]} + \varepsilon_{r_d}$$

$$\sigma_{\varepsilon_{r_d}} = \begin{cases} d^{0.85} \cdot 0.0198 & \text{if } d \leq 12\text{m} \\ 12^{0.85} \cdot 0.0198 & \text{if } d > 12\text{m} \end{cases}$$



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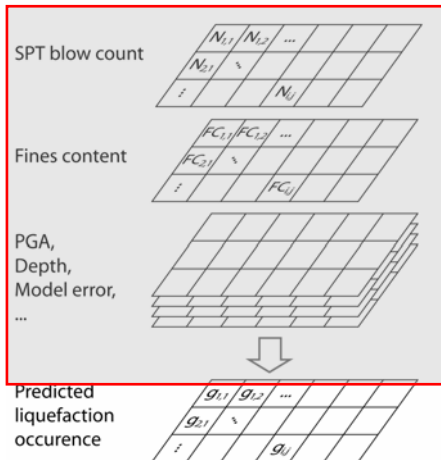
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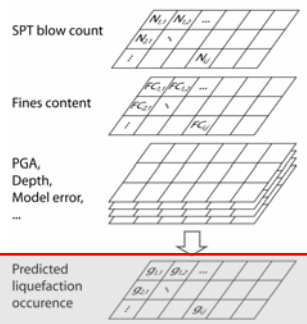
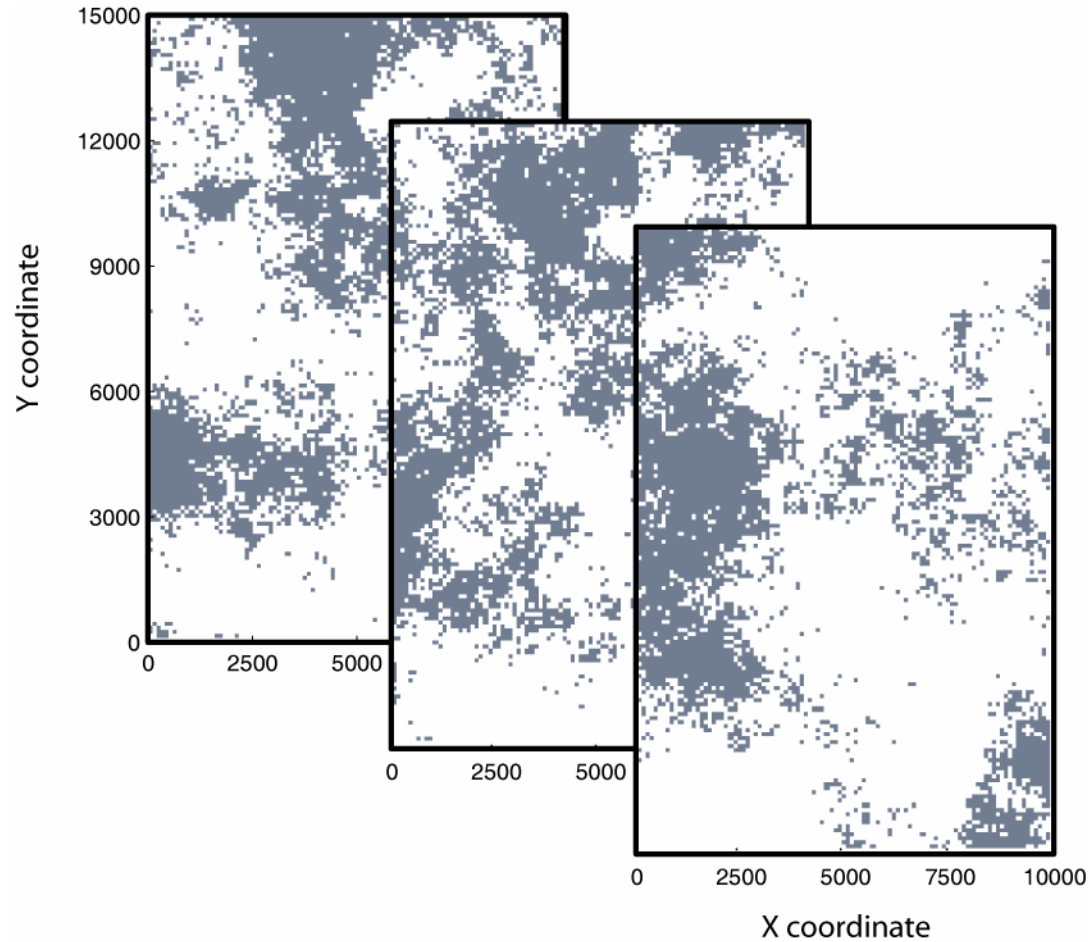
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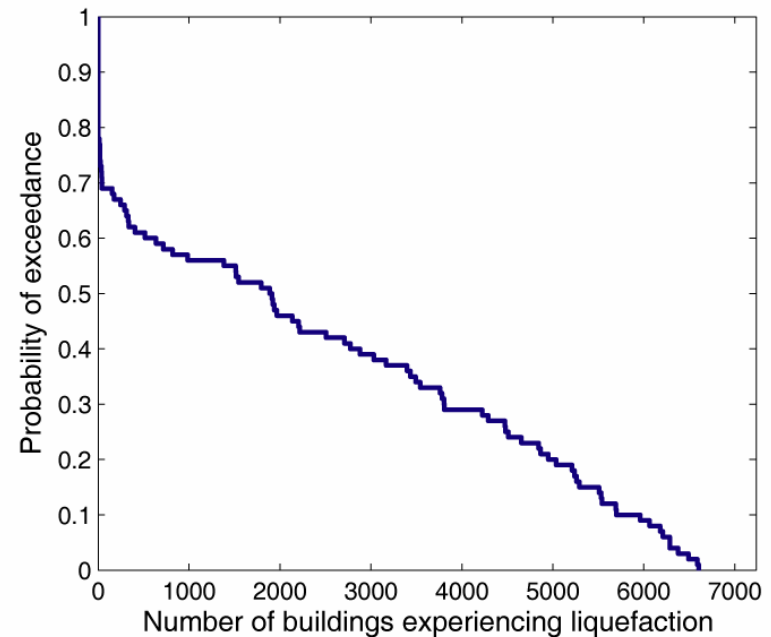
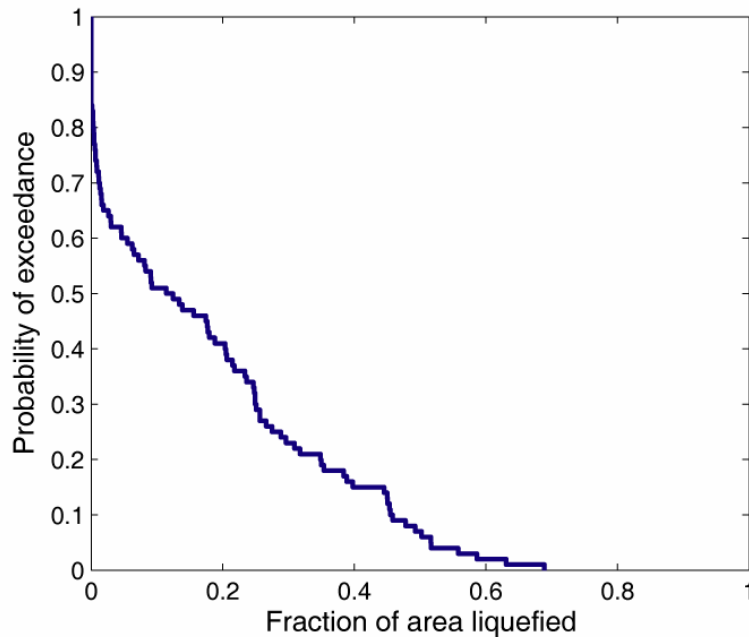


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# Realizations of liquefaction extent

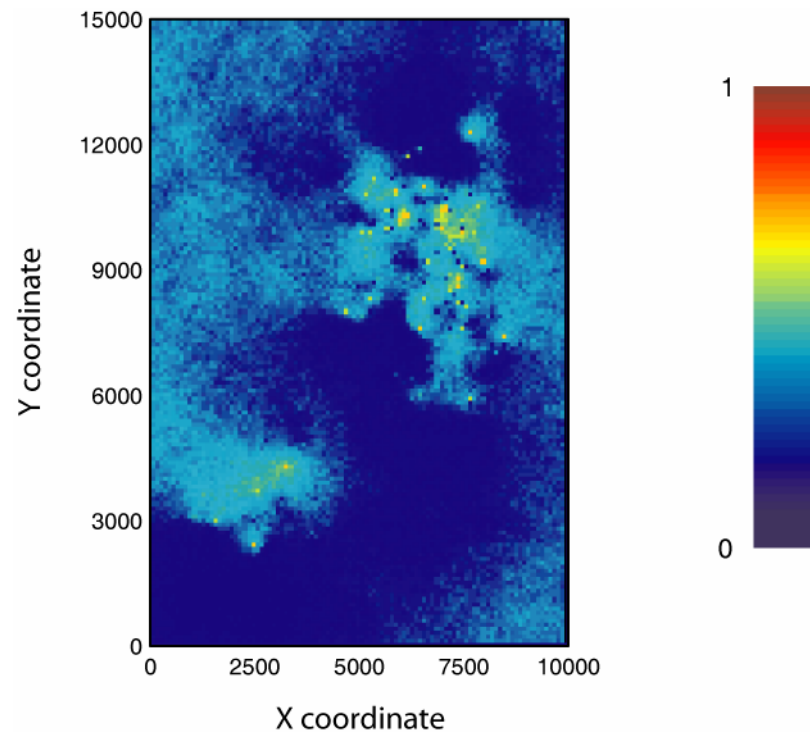


## Probabilistic evaluation of consequences, given a M=7 earthquake causing a PGA of 0.3g

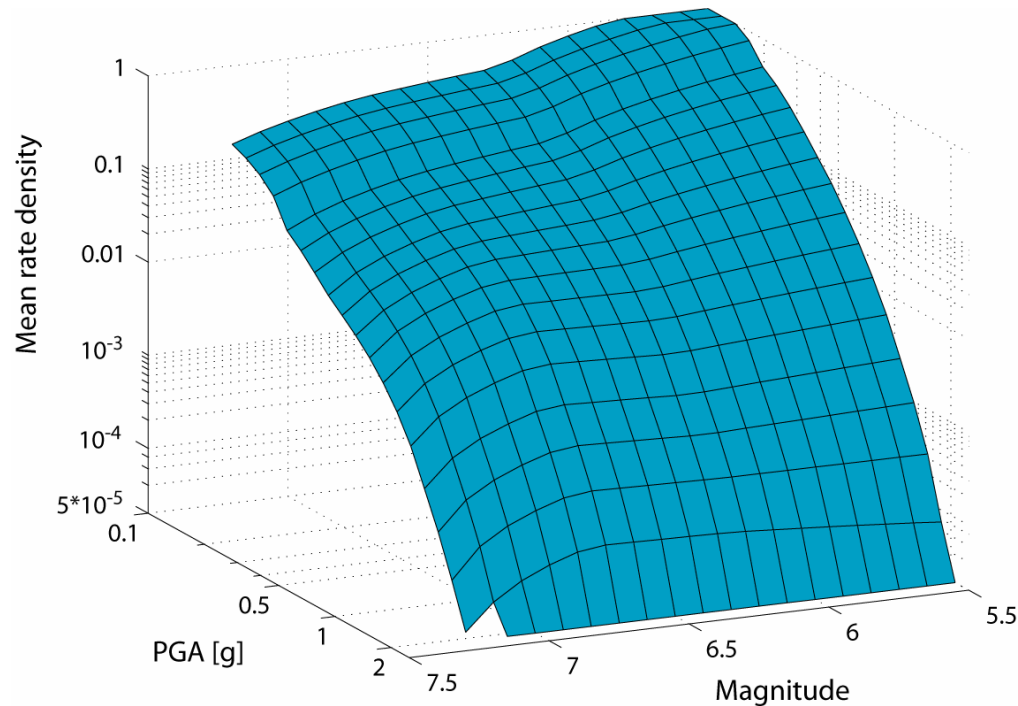


$$P(Y > y | pga, m) = \int_{\eta} I(h(\mathbf{g}(\boldsymbol{\eta} | pga, m) > y)) f_{\boldsymbol{\eta}}(\mathbf{e}) d\mathbf{e}$$

# Probability of liquefaction at the study site, given a $M=7.4$ earthquake causing a PGA of $0.3g$



The previously computed result for a deterministic loading case can be combined with stochastic loading obtained from probabilistic seismic hazard analysis (PSHA)



## Comments

- The procedure is useful for organizing the many pieces of relevant engineering data, but the inputs need careful consideration
- Soil random field models are challenging to characterize
  - Soil layering?
  - Unidentified local soil lenses?
  - Homogeneity/Ergodicity assumptions?
- Modeling post-liquefaction behavior is a challenge
- Geotechnical data often comes from several sources of varying quality

## Conclusions

- A framework has been proposed for modeling the spatial extent of liquefaction
  - Accounts for spatial dependence of soil properties
  - Incorporates known values of soil properties at sampled locations
  - Complex functions of the spatial extent of liquefaction can be evaluated
- Probabilistic seismic hazard analysis can be used to incorporate all possible ground motion intensities
  - Avoids the use of a scenario load intensity with unknown recurrence rate
  - Provides an explicit estimate of annual occurrence probabilities
- This approach potentially allows for the design of projects with uniform levels of reliability